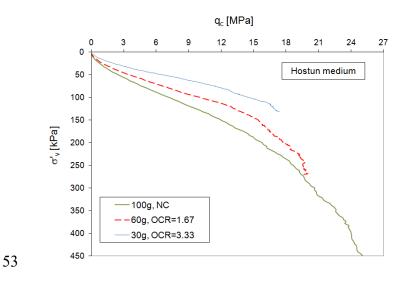
1 2 3 4	This material may be downloaded for personal use only. Any other use requires prior permission of the American Society of Civil Engineers. This material may be found at https://ascelibrary.org/doi/10.1061/%28ASCE%29GT.1943-5606.0002411
5 6 7 8 9 10	Postprint of: Bałachowski L., Discussion of "CPT Evaluation of Yield Stress Profiles in Soils" by Shehab S. Agaiby and Paul W. Mayne, JOURNAL OF GEOTECHNICAL AND GEOENVIRONMENTAL ENGINEERING, Vol. 146, iss. 12 (2020), 07020022, https://doi.org/10.1061/(ASCE)GT.1943-5606.0002411
10	Discussion of "CPT evaluation of yield stress profiles in soils"
12	by Shehab S. Agaiby and Paul W. Mayne
13	
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17	
18	The Authors presented a comprehensive approach concerning the estimation of yield stress
19	profile in different soils. They considered a large database including worldwide well-
20	documented test sites and calibration chamber tests. I would like to focus the discussion on the
21	evaluation of OCR ratio in sands in the continuous mini-cone penetration using the results of
22	centrifuge tests. Physical modelling in centrifuge permits to perform some parametric studies
23	including the effect of soil overconsolidation. The aim of this discussion is to verify the
24	correlations for OCR in sands - proposed by the Authors - in case of defined uniform OCR
25	within the soil profile. A series of centrifuge CPT tests (Bałachowski, 1995) was conducted in
26	dense D _R =0.82 uniform quartz sand using mini-CPT model (B=12 mm). The soil mass with
27	medium Hostun sand ($d_{50}=0.32$ mm) was prepared using sand raining technique. The
28	overconsolidated soil mass was obtained by reducing the centrifuge g-level from 100g to 60g
29	or 30g (with overconsolidation ratio, OCR=1.67 or 3.33, respectively). In this way a uniform
30	OCR ratio was achieved in the soil profile. The mini-cone penetration tests were conducted in

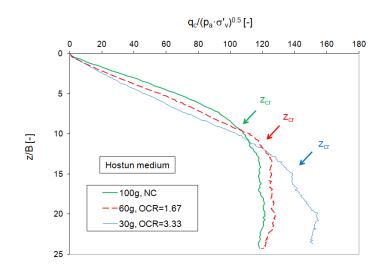
normally consolidated sand, i.e. at 100g and in overconsolidated sand after reduction of
centrifuge acceleration to 60g (OCR=1.67) and then to 30g (OCR=3.33).

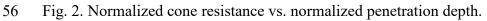
33 The results of cone resistance in Hostun medium sand are shown in Fig. 1. One can notice that 34 at a given vertical stress higher cone resistance is mobilized in overconsolidated soil mass. In 35 case of shallow penetration scheme the normalized cone resistance increases almost linearly 36 with vertical stress (Fig. 2). It can be also noticed that the critical depth increases with OCR 37 ratio. Calibration chamber tests (Jamiolkowski et al. 1985) have shown that under critical depth 38 the cone resistance is proportional to the square root of vertical stress. It was also confirmed in 39 cone penetration tests in centrifuge (Gui et al. 1998, Bolton et al. 1999, Bałachowski, 2007, 40 Salgado, 2014 and Kim et al. 2015).

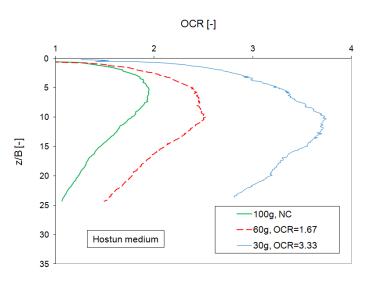
41 The OCR ratio for cone penetration tests (Fig. 3) was calculated using the Eq. 7 proposed by 42 the Authors. In case of shallow penetration mode the calculated OCR values increase with 43 vertical stress and reach the maximum value near the critical depth, and then attenuate with 44 further penetration. The calculated OCR is closer to the simulated value at large penetration 45 depths (vertical stress). It is interesting to note that for normally consolidated sand the 46 calculated OCR values are higher than one and similar shape of calculated OCR profile is 47 observed in normally consolidated and overconsolidated sands. The general observation is that 48 the proposed formula for yield stress (Eq. 7) can be used only in case of deep penetration 49 scheme and seems to overestimate OCR ratio in sands. If the grain size effects is rather 50 negligible (Gui et al. 1998) in the considered tests ($B/d_{50}=37.5$) the geometrical size effects 51 could however influence the results at lower stress level. Further studies including the procedure 52 "modelling of models" will be necessary to study this effect.



54 Fig. 1. Mini-cone penetration tests in Hostun medium sand.







58 Fig. 3. OCR ratio derived from mini-cone penetration tests.

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60 References

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- 62 Bałachowski, L. (1995). Différents aspects de la modélisation physique du comportement des
- 63 pieux: Chambre d'Etalonnage et Centrifugeuse, Thèse de doctorat, Institut National
- 64 Polytechnique de Grenoble, France, 320 pp.
- Bałachowski, L. (2007). "Size effect in centrifuge cone penetration tests." *Archives of Hydro- Engineering and Environmental Mechanics*, Vol. 54, No.3, pp. 161-181.
- 67 Bolton, M. D., Gui, M. W., Garnier, J., Corté, J. F., Bagge, G., Laue, J., and Renzi, R. (1999).
- 68 "Centrifuge cone penetration tests in sand." *Géotechnique*, 49 (4), pp. 543-552.
- 69 Gui, M. W., Bolton, M. D., Garnier, J., Corté, J. F., Bagge, G., Laue, J., and Renzi, R. (1998).
- 70 "Guidelines for cone penetration tests in sand." Proc. Int. Conf. on Centrifuge Modelling
- 71 (Centrifuge'98), Tokyo, Kimura et al. (Ed.), Vol. 1, A.A. Balkema, Rotterdam, Netherlands,
 72 pp. 155-160.
- Jamiolkowski, M., Ladd, C. C., Germaine, J. T., and Lancellotta, R. (1985). "New
 developments in field and laboratory testing of soils." *Proc., XI ICSMFE*, Vol. 1, A.A.
 Balkema, Rotterdam, Netherlands, 57-153.
- Kim, J. H., Choo, Y. W., Kim, D. J., and Kim, D. S. (2015). "Miniature cone tip resistance on
 sand in a centrifuge." *J. Geotech. Geoenviron. Eng.*, 142 (3), 10.1061/(ASCE)GT.19435606.0001425.
- Salgado, R. (2014). "Experimental research on cone penetration resistance." In *Proc., Geo- Congress 2014 Keynote Lectures*, 140-163. Reston, VA:ASCE.