

Numerical evaluation of dynamic response of an experimentally tested base-isolated and fixed-base steel structure model

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ABSTRACT: Seismic isolation is recognized as one of the most popular and effective methods of protecting structures during earthquake. The present paper is focused on the comparison between the dynamic responses of buildings with fixed and isolated bases exposed to seismic excitations. The aim of the study is to investigate the effectiveness of a simplified base isolation numerical modelling technique using the linear springs. One-storey steel structure model has been considered using ETABS software, which was previously tested during the shaking table tests. The comparison of the structural response has been performed between the numerical and experimental results. High compatibility of the results for the fixed-base steel model between the numerical and experimental analyses has been obtained which proves the effectiveness of the modelling techniques used in the ETABS software. It has also been found that the isolation system is effective in improving the response of the steel structure during earthquake. However, modelling the isolation system using the linear springs is not an accurate technique due to the large differences in the structural responses obtained between the numerical and experimental analyses.

1 INTRODUCTION

Earthquake is considered as one of the most dangerous phenomenon caused by nature as it led to catastrophic damages in many previous seismic events. Scientists have worked extensively to develop new techniques to decrease the level of damages of buildings exposed to earthquakes or eliminate the damages at all (see, for example, Sołtysik, Falborski & Jankowski, 2016; Sołtysik, Falborski & Jankowski, 2017; Miari, Choong & Jankowski, 2019; Naderpour, Naji, Burkacki & Jankowski, 2019; Miari, Choong & Jankowski, 2020). One of the modern techniques to resist the effects of earthquakes is the use of isolation systems (see, for example, Robinson & Greenbank, 1976; Robinson, 1982; Buckle & Mayes, 1990; Kelly, 1990; Kelly, 1993; Skinner, Robinson & McVerry, 1993; Robinson, 1998; Buckle, 2000). Base isolators, such as Lead Rubber Bearings, High Damping Rubber Bearings, and Friction Pendulum Bearings, are widely used in practice in many earthquake-prone regions to mitigate structural vibrations, and consequently minimize the loss of life and property damage during seismic events (see, for example, Nagarajaiah & Xiaohong, 1996; Nagarajaiah & Xiaohong, 2000). These systems decrease the damages in buildings during earthquakes by controlling the accelerations and displacements of the vibrating structures through inducing flexibility to their bases. The use of this technique prevents the resonance between the structure and the dominant frequency of the ground motion (see, for example, Booth & Key, 2006; Chopra, 2012).

In order to get an insight into the mechanical behaviour of base isolation devices, quasi-static or dynamic tests are usually performed. One of the most common procedures for experimentally testing buildings subjected to earthquakes is to test them on a seismic shaking table

(see, for example, Falborski & Jankowski, 2017a). Also, the dynamic behaviour of a structure is often modelled numerically using commercial software (see, for example, Falborski & Jankowski, 2017b; Falborski, Sołtysik & Jankowski, 2018; Lasowicz, Falborski & Jankowski, 2018; Falborski & Lasowicz, 2019). This facilitates the consideration of highly complex problems with less difficulties and budget. In the numerical analysis of buildings, several factors are important, such as the modelling approach and the ground conditions. The ground conditions have a large impact on the response of the structure under a seismic load (see, for example, Lasowicz & Falborski, 2018; Falborski, 2020, Falborski, 2020, Elwardany, Selemah, Jankowski & El-khoriby, 2019).

The aim of this study is to investigate the effectiveness of a simplified base isolation modelling technique using the linear springs since the advanced mathematical models are often difficult to be applied in commercial programs. One-storey steel structure model has been considered using ETABS software, which was previously tested during a seismic table test (see, for example, Falborski & Jankowski, 2018). The response of the numerical model of the one-storey building has been compared with that of the experimental model intending to validate the numerical model. The steel structure model has been studied with two types of bases, i.e. fixed and isolated bases. Comparison has been performed between these models (with different types of bases) to investigate the effect of the use of isolation system on the response of buildings exposed to earthquake excitations.

2 EXPERIMENTAL MODEL AND SHAKING TABLE INVESTIGATION

The experimental study has been conducted for a single-storey steel structure model. This model, consisting of a steel frame and two concrete slabs, is of 1.20 m height and weights 95.12 kg (see Figure 1). Rectangular elements made of hollow section elements (RHS 15×15×1.5 mm) form a welded steel frame, and the columns have been set on a rectangular plan with a spacing of 0.556 m in the transverse direction (x-direction) and 0.465 m in the longitudinal direction (y-direction). Diagonal bracings have also been used in the planes of the sidewalls, which are responsible for counteracting transverse and torsional vibrations. The simulation of the weight of the floor and foundation slabs is presented through two concrete plates (50×50×7 cm).

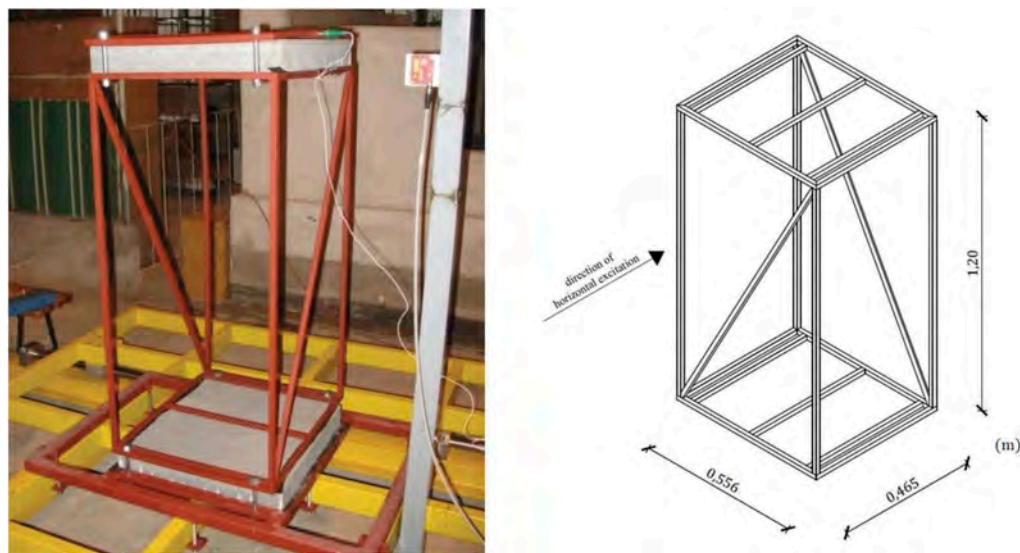


Figure 1. Single-storey steel structure model mounted on the shaking table.

The dynamic characteristics of the experimental model were previously determined by conducting free vibration tests (see Falborski & Jankowski, 2017a). The fundamental frequency of the experimental model was calculated to be 3.31 Hz, whereas the damping ratio 0.53%. A middle-sized shaking table located at Gdańsk University of Technology, Poland, was used to investigate the seismic response of the experimental model to a series of earthquake ground motions. All the experimental tests for steel structure models were carried out for a time step size of 0.002 s.

3 NUMERICAL MODEL

In this study, single-storey building has been modelled in ETABS software using the Finite Element (FE) method. The slabs have been modelled using shell elements and the beams and columns applying frame elements. The numerical model has been studied first with fixed bases. Then, the same model has been studied with the isolated bases. A simple mathematical model in the form of springs has been introduced in ETABS software. The FE models of both building (with fixed and isolated bases) are shown in Figure 2.

To determine the lateral stiffness K of the springs used, the hysteresis loop at excitation of 2 Hz has been utilized (see Figure 3). By using this method, the value of the lateral stiffness has been found to be 45 kN/m.

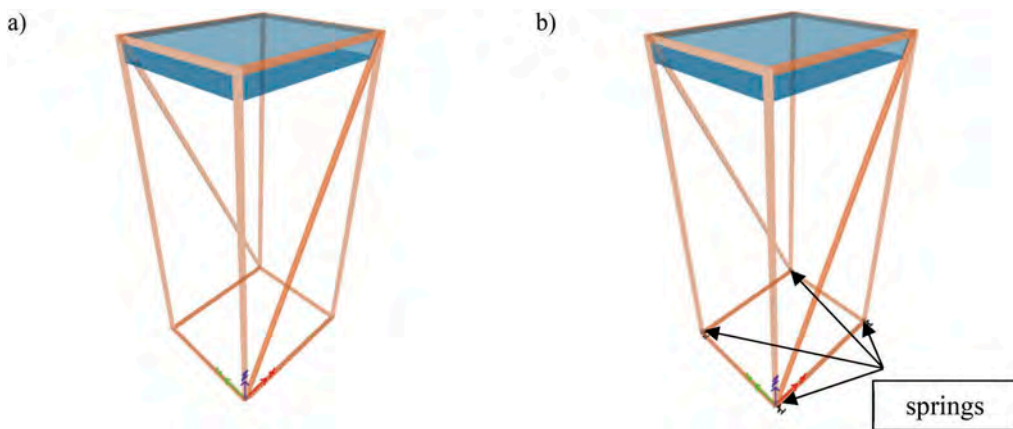


Figure 2. Numerical models of fixed-base building a) and base-isolated building b).

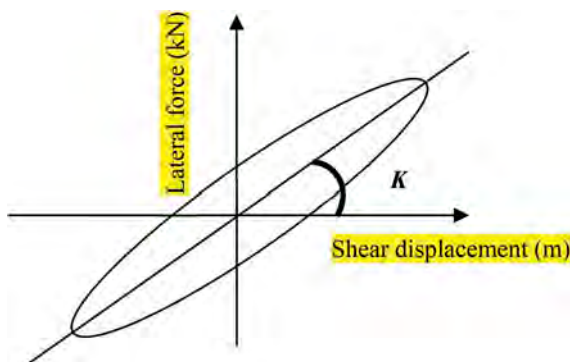


Figure 3. Schematic diagram of a hysteresis loop.

Table 1. Ground motions used in this study.

Earthquake	PGA [m/s^2]	Station	Year
El Centro	3.070	Array 9	1940
San Fernando	5.688	Paicoma Dam	1971

The detailed numerical analyses have been performed for the described model under two ground motions with different Peak Ground Accelerations (PGA) (see Table 1 for details). The response has been obtained using the linear direct integration method which is based on the Hilbert-Hughes-Taylor method, where the leading parameters are $\gamma = 0.5$, $\beta = 0.25$ and $\alpha = 0$. The time step size used for the El Centro and San Fernando ground motions has been set at 0.01 s. All the mentioned earthquakes have been properly scaled in the ETABS software to obtain comprehensive comparative analysis.

4 VALIDATION OF THE NUMERICAL MODEL

The acceleration time histories computed for the fixed-base model and base-isolated model under various seismic excitations are presented in Figures 4-5. The comparison of the results obtained from the numerical analysis and the shaking table investigation are briefly reported in Table 2 and Table 3. The comparison between the numerical and experimental models for the fixed-base buildings reveals that there is a good agreement between the responses. The difference of the peak acceleration between the numerical and experimental models of the fixed-base building ranges between 4% and 11% (see Figure 4 and Table 2). This means that the numerical model is capable of representing the response of the fixed-base building quite accurately. However, the comparison between the acceleration time histories of the numerical and experimental models for the base-isolated buildings reveals that the response is not close and there is no good agreement between them. The difference of the peak acceleration between the

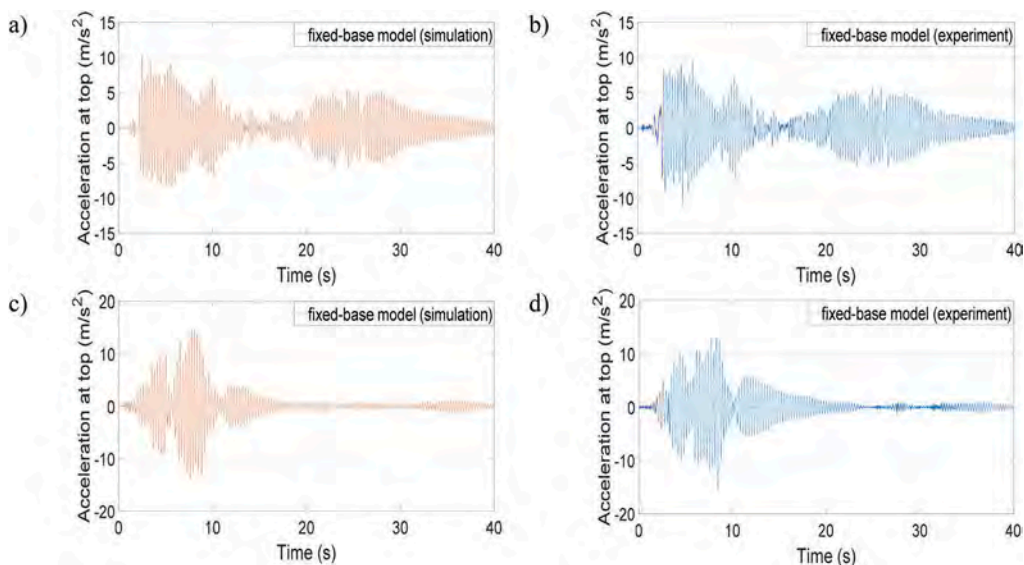


Figure 4. Numerically obtained (left) and experimentally determined (right) time-acceleration history plots for the fixed-base one-storey model during different ground motions: a, b) the 1940 El Centro earthquake and c, d) the 1971 San Fernando earthquake.

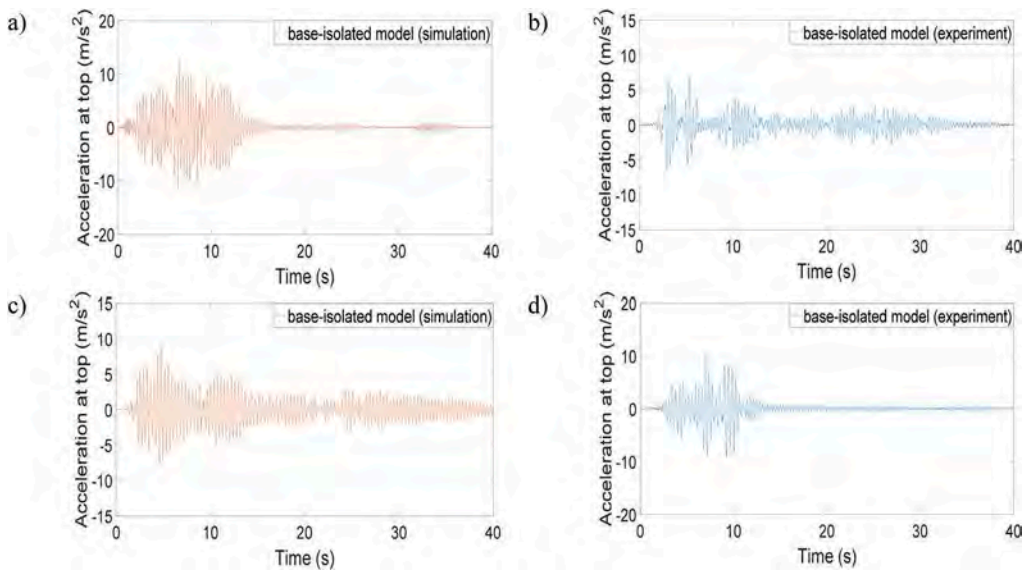


Figure 5. Numerically obtained (left) and experimentally determined (right) time-acceleration history plots for the base-isolated one-storey model during different ground motions: a, b) the 1940 El Centro earthquake and c, d) the 1971 San Fernando earthquake.

Table 2. Results obtained from numerical and experimental investigation for fixed-base model.

Dynamic excitation	Peak acceleration at the top of the single-storey steel structure model [m/s ²]		
	Numerical analysis using ETABS	Shaking table investigation	Percentage difference [%]
El Centro earthquake	10.21	11.39	10.36
San Fernando earthquake	14.96	15.59	4.04

Table 3. Results obtained from numerical and experimental investigation for base-isolated model.

Dynamic excitation	Peak acceleration at the top of the single-storey steel structure model [m/s ²]		
	Numerical analysis using ETABS	Shaking table investigation	Percentage difference [%]
El Centro earthquake	9.06	6.98	22.96
San Fernando earthquake	12.89	10.47	18.77

numerical and experimental models of the base-isolated building ranges between 19% and 23% (see Figure 5 and Table 3). This means that the numerical model is not capable of representing the response of the base-isolated building. This is referred to the mathematical simplification of the spring model. For more accurate results, more advanced bearing model should be used.

5 CONCLUSIONS

This paper has been focused on comparison between the dynamic responses of buildings with fixed and isolated bases exposed to earthquake excitations. One-storey steel structure model has been considered using ETABS software, which was previously tested during the shaking table tests. The main conclusions of this study are:

- High compatibility of the results for the fixed-base steel structure model between the numerical and experimental analyses has been obtained which proves the effectiveness of the modelling techniques used in the ETABS software.
- The use of isolated system is an effective method in improving the response of building exposed to earthquake excitations, as concluded from the results of both experimental and numerical analyses.
- Modelling the isolated system by using the simplified method with linear springs is not an accurate technique due to the large differences in the structural responses found between the numerical and experimental analyses. Therefore, more advanced mathematical models should be used to simulate the behaviour of isolation systems during earthquakes.

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